APPENDIX B:

SOIL BORINGS

Kane, Kathleen

From: Mike Schultz <mschultz@cgcinc.net>
Sent: Sunday, October 31, 2021 1:44 PM
To: Kane, Kathleen
Cc: 'Eric Fair'
Subject: Warner Park Crayon Playground Facility C21051-13
Attachments: Warner Park Crayon Playground Location Map.pdf; Warner Park Crayon Playground Borings.pdf

At your request, CGC completed four soil borings where a new fully accessible playground facility is planned. We understand that the playground facilities will use concrete footings founded at least 4 ft below final grades for frost protection. It is also desired to relocate a basketball court to this area, with asphalt pavement to be used. The borings were done by Badger State Drilling (under subcontract to CGC) on October 15, 2021 near the locations selected by City of Madison personnel (location map attached). CGC staked out the boring locations. The soil profile revealed in the borings is fairly consistent with other borings done within Warner Park and involves the following (in descending order):

- 6 to 13 in. of topsoil
- About 2 to 2.5 ft of fill consisting of medium dense sands and/or stiff clays
- Approximately 1.5 to 2.5 ft of very loose to loose peat (absent at Boring 4)
- Roughly 2 to 4.5 ft of soft to medium stiff clay
- Followed by loose to medium dense sand with varying silt content to the maximum depths explored.

Groundwater was observed in each boring to a depth as shallow of 5.8 ft. Note that groundwater is a function of the lake level of nearby Lake Mendota and other factors such as precipitation, etc. Additional soil and groundwater details are presented on the attached boring logs.

In our opinion, the peats observed at a typical footing bearing depth of 4 ft are NOT acceptable for footing support. Instead we recommend that footings be founded within the medium stiff gray clay encountered at a depth of 5.5 to 6 ft, with strip footings to be a minimum of 18-in. wide and column pads a minimum of 24-in. square. Footing subgrades should be cut with a smooth-edged bucket to minimize disturbance. In addition, since groundwater likely will be observed during footing construction, we recommend that a minimum 6-in. layer of clear stone or 3-in. dense graded base (BGD) be placed below footings that is compacted until deflection ceases. This will allow water to be pumped and removed if observed, with means and methods the contractor responsibility to create “construction in the dry” as much as practical. A relatively low allowable design soil bearing pressure of 750 psf is recommended for footing sizing. This design soil bearing pressure should be used for possible playground equipment if a drilled shaft option is implemented. Again the shafts should extend below the peat into the gray clays.

If concrete slabs are to be built for the facilities, they can be founded on the observed fill (after topsoil removal and provided it proof-rolls satisfactorily) and designed assuming a subgrade modulus of 100 pci. If proof-rolling using a heavy piece of construction equipment indicates soft conditions then those fill materials should be removed and replaced with 3-in. DGB that is compacted until deflection ceases. We recommend that a 6-in. layer of bedding material be placed below the concrete slab involving granular soils having a P200 content of less than 5%. If asphalt pavement is to be used, which is planned for the basketball court, we recommend it be at least 3.5-in. thick underlain by 12-in. of compacted base course. Additional details can be provided upon request.

It should be noted that the recommendations described above assume that final design slab/pavement grades will match existing grades and no new fill will be placed. Should fill be needed that is 1 ft thick or more, we recommend that a period of approximately 6 months be implemented before slab/pavement placement to allow for the highly

compressible peats to consolidate. Monitoring should be accomplished using surveyed settlement points to evaluate that consolidation/settlement is tapering off.

We trust this brief report addresses your present needs. Please contact CGC if we can be of further service or should questions develop upon review of this transmittal. Information regarding limitations pertaining to opinions presented in this submittal is attached. Thank you.

Michael N. Schultz, P.E.
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Madison, WI 53713
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Web Site: www.cgcinc.net

CGC Inc.

LOG OF TEST BORING

Project: Warner Park Crayon Playground

Location: Madison, WI

Boring No. 1
Surface Elevation (ft) 854±
Job No. C21051-13
Sheet 1 of 1

2921 Perry Street, Madison, WI 53713 (608) 288-4100, FAX (608) 288-7887

SAMPLE No.	TYPE	Rec (in.)	Moist	N	Depth (ft)	VISUAL CLASSIFICATION and Remarks	SOIL PROPERTIES qu (qa) (tsf)	W	LL	PL	LI
						13 in. TOPSOIL					
1		16	M	10		FILL: Stiff Brown Clay	(1.5)				
						Loose to Very Loose, Black Sedimentary to Fibrous PEAT (PT)					
2		12	M	4							
					5						
						Medium Stiff, Gray Lean CLAY, Trace Sand (CL)					
3		17	M/W	2			(0.75)				
4		16	M/W	10		Sandy Near 8.5'	(0.6)				
					10	Loose to Medium Dense, Gray Fine to Medium SAND, Little to Some Silt (SP-SM/SM)					
5		16	W	12							
						End of Boring at 12.5 ft Backfilled with Bentonite Chips and Turf Plug					
					15						

WATER LEVEL OBSERVATIONS

While Drilling ▽ 7.0' Upon Completion of Drilling 6.5'
Time After Drilling
Depth to Water ▼
Depth to Cave in 11.1'

The stratification lines represent the approximate boundary between soil types and the transition may be gradual.

GENERAL NOTES

Start 10/15/21 End 10/15/21
Driller SE Chief CRJ Rig 7822DT
Logger Ben Editor ESF
Drill Method 2.25" HSA; Autohammer

CGC Inc.

LOG OF TEST BORING

Project: **Warner Park Crayon Playground**

Location: **Madison, WI**

Boring No. **2**
Surface Elevation (ft) **854±**
Job No. **C21051-13**
Sheet **1** of **1**

2921 Perry Street, Madison, WI 53713 (608) 288-4100, FAX (608) 288-7887

SAMPLE No.	TYPE	Rec (in.)	Moist	N	Depth (ft)	VISUAL CLASSIFICATION and Remarks	SOIL PROPERTIES qu (qa) (tsf)	W	LL	PL	LI
						7 in. TOPSOIL					
1		12	M	11		FILL: Mixed Medium Dense Sand and Stiff Clay with Gravel	(1.5+)				
						Very Loose, Black Sedimentary to Fibrous PEAT (PT)					
2		15	M	2							
					5	Medium Stiff, Gray Lean CLAY, Trace Sand, Scattered Thin (<1/2 in.) Sandy Seams (CL)					
3		16	M	4			(0.75)				
					▽						
4		15	M/W	9			(0.6)				
					10	Loose, Gray Fine to Medium SAND, Little to Some Silt (SP-SM/SM)					
						Medium Dense, Gray-Brown Fine to Coarse SAND, Some Gravel, Trace Silt (SP)					
5		17	W	18							
						End of Boring at 12.5 ft Backfilled with Bentonite Chips and Turf Plug					
					15						

WATER LEVEL OBSERVATIONS

While Drilling ▽ **8.5'** Upon Completion of Drilling **6.6'**
Time After Drilling
Depth to Water ▼
Depth to Cave in **8.7'**

The stratification lines represent the approximate boundary between soil types and the transition may be gradual.

GENERAL NOTES

Start **10/15/21** End **10/15/21**
Driller **SE** Chief **CRJ** Rig **7822DT**
Logger **Ben** Editor **ESF**
Drill Method **2.25" HSA; Autohammer**

CGC Inc.

LOG OF TEST BORING

Project: Warner Park Crayon Playground

Location: Madison, WI

Boring No. 3
Surface Elevation (ft) 855±
Job No. C21051-13
Sheet 1 of 1

2921 Perry Street, Madison, WI 53713 (608) 288-4100, FAX (608) 288-7887

SAMPLE No.	TYPE	Rec (in.)	Moist	N	Depth (ft)	VISUAL CLASSIFICATION and Remarks	SOIL PROPERTIES qu (qa) (tsf)	W	LL	PL	LI
						6 in. TOPSOIL					
1		17	M	14		FILL: Medium Dense Brown and Gray Silty Sand with Clay and Gravel					
2		12	M	4	5	Loose to Very Loose, Black Sedimentary to Fibrous PEAT (PT)					
3		17	M	2		Medium Stiff to Soft, Gray Lean CLAY, Trace Sand, Scattered Plant Fibers (CL)	(0.5)				
4		16	W	8	10	Loose, Gray Fine to Medium SAND, Little Silt (SP-SM)					
5		17	W	18		Medium Dense, Gray-Brown Fine to Coarse SAND, Some Gravel, Trace Silt (SP)					
					15	End of Boring at 12.5 ft Backfilled with Bentonite Chips and Turf Plug					

WATER LEVEL OBSERVATIONS

While Drilling ▽ 9.0' Upon Completion of Drilling 5.8'
Time After Drilling
Depth to Water ▼
Depth to Cave in 7.1'

The stratification lines represent the approximate boundary between soil types and the transition may be gradual.

GENERAL NOTES

Start 10/15/21 End 10/15/21
Driller SE Chief CRJ Rig 7822DT
Logger Ben Editor ESF
Drill Method 2.25" HSA; Autohammer

CGC Inc.

LOG OF TEST BORING

Project: Warner Park Crayon Playground

Location: Madison, WI

Boring No. 4
Surface Elevation (ft) 855±
Job No. C21051-13
Sheet 1 of 1

2921 Perry Street, Madison, WI 53713 (608) 288-4100, FAX (608) 288-7887

SAMPLE No.	TYPE	Rec (in.)	Moist	N	Depth (ft)	VISUAL CLASSIFICATION and Remarks	SOIL PROPERTIES qu (qa) (tsf)	W	LL	PL	LI
						7 in. TOPSOIL					
						FILL: Mixed Medium Dense Brown Silty Sand and Stiff Clay					
1		16	M	11			(1.5)				
						Medium Stiff, Gray Lean CLAY, Trace Sand (CL)					
2		16	M	4			(0.8)				
					5						
3		17	W	1		Becomming Sandy and Very Soft Near 6 ft	(<0.2)				
						Loose, Gray Fine SAND, Some Silt (SM)					
4		15	W	9							
					10						
5		16	W	22							
						Medium Dense, Gray-Brown Fine to Coarse SAND, Some Gravel, Little Silt (SP-SM)					
						End of Boring at 12.5 ft					
						Backfilled with Bentonite Chips and Turf Plug					
					15						

WATER LEVEL OBSERVATIONS

While Drilling ▽ 7.0' Upon Completion of Drilling 6.9'
Time After Drilling
Depth to Water ▼
Depth to Cave in 8.4'

The stratification lines represent the approximate boundary between soil types and the transition may be gradual.

GENERAL NOTES

Start 10/15/21 End 10/15/21
Driller SE Chief CRJ Rig 7822DT
Logger Ben Editor ESF
Drill Method 2.25" HSA; Autohammer

CGC, Inc.

LOG OF TEST BORING
General Notes

DESCRIPTIVE SOIL CLASSIFICATION

Grain Size Terminology

Soil Fraction	Particle Size	U.S. Standard Sieve Size
Boulders	Larger than 12"	Larger than 12"
Cobbles	3" to 12"	3" to 12"
Gravel: Coarse	¾" to 3"	¾" to 3"
Fine	4.76 mm to ¾"	#4 to ¾"
Sand: Coarse	2.00 mm to 4.76 mm	#10 to #4
Medium	0.42 to mm to 2.00 mm	#40 to #10
Fine	0.074 mm to 0.42 mm	#200 to #40
Silt	0.005 mm to 0.074 mm	Smaller than #200
Clay	Smaller than 0.005 mm	Smaller than #200

Plasticity characteristics differentiate between silt and clay.

General Terminology

Physical Characteristics
Color, moisture, grain shape, fineness, etc.
Major Constituents
Clay, silt, sand, gravel
Structure
Laminated, varved, fibrous, stratified, cemented, fissured, etc.
Geologic Origin
Glacial, alluvial, eolian, residual, etc.

Relative Density

Term	"N" Value
Very Loose	0 - 4
Loose	4 - 10
Medium Dense	10 - 30
Dense	30 - 50
Very Dense	Over 50

Relative Proportions Of Cohesionless Soils

Proportional Term	Defining Range by Percentage of Weight
Trace	0% - 5%
Little	5% - 12%
Some	12% - 35%
And	35% - 50%

Consistency

Term	q_u-tons/sq. ft
Very Soft	0.0 to 0.25
Soft	0.25 to 0.50
Medium	0.50 to 1.0
Stiff	1.0 to 2.0
Very Stiff	2.0 to 4.0
Hard	Over 4.0

Organic Content by Combustion Method

Soil Description	Loss on Ignition
Non Organic	Less than 4%
Organic Silt/Clay	4 – 12%
Sedimentary Peat	12% - 50%
Fibrous and Woody Peat	More than 50%

Plasticity

Term	Plastic Index
None to Slight	0 - 4
Slight	5 - 7
Medium	8 - 22
High to Very High	Over 22

The penetration resistance, N, is the summation of the number of blows required to effect two successive 6" penetrations of the 2" split-barrel sampler. The sampler is driven with a 140 lb. weight falling 30" and is seated to a depth of 6" before commencing the standard penetration test.

SYMBOLS

Drilling and Sampling

CS – Continuous Sampling
RC – Rock Coring: Size AW, BW, NW, 2"W
RQD – Rock Quality Designation
RB – Rock Bit/Roller Bit
FT – Fish Tail
DC – Drove Casing
C – Casing: Size 2 ½", NW, 4", HW
CW – Clear Water
DM – Drilling Mud
HSA – Hollow Stem Auger
FA – Flight Auger
HA – Hand Auger
COA – Clean-Out Auger
SS - 2" Dia. Split-Barrel Sample
2ST – 2" Dia. Thin-Walled Tube Sample
3ST – 3" Dia. Thin-Walled Tube Sample
PT – 3" Dia. Piston Tube Sample
AS – Auger Sample
WS – Wash Sample
PTS – Peat Sample
PS – Pitcher Sample
NR – No Recovery
S – Sounding
PMT – Borehole Pressuremeter Test
VS – Vane Shear Test
WPT – Water Pressure Test

Laboratory Tests

q_a – Penetrometer Reading, tons/sq ft
q_a – Unconfined Strength, tons/sq ft
W – Moisture Content, %
LL – Liquid Limit, %
PL – Plastic Limit, %
SL – Shrinkage Limit, %
LI – Loss on Ignition
D – Dry Unit Weight, lbs/cu ft
pH – Measure of Soil Alkalinity or Acidity
FS – Free Swell, %

Water Level Measurement

▽ - Water Level at Time Shown
NW – No Water Encountered
WD – While Drilling
BCR – Before Casing Removal
ACR – After Casing Removal
CW – Cave and Wet
CM – Caved and Moist

Note: Water level measurements shown on the boring logs represent conditions at the time indicated and may not reflect static levels, especially in cohesive soils.

CGC, Inc.

Madison - Milwaukee

Unified Soil Classification System

UNIFIED SOIL CLASSIFICATION AND SYMBOL CHART

COARSE-GRAINED SOILS (more than 50% of material is larger than No. 200 sieve size)			
GRAVELS More than 50% of coarse fraction larger than No. 4 sieve size	Clean Gravels (Less than 5% fines)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines
		GP	Poorly-graded gravels, gravel-sand mixtures, little or no fines
	Gravels with fines (More than 12% fines)	GM	Silty gravels, gravel-sand-silt mixtures
		GC	Clayey gravels, gravel-sand-clay mixtures
SANDS 50% or more of coarse fraction smaller than No. 4 sieve size	Clean Sands (Less than 5% fines)	SW	Well-graded sands, gravelly sands, little or no fines
		SP	Poorly graded sands, gravelly sands, little or no fines
	Sands with fines (More than 12% fines)	SM	Silty sands, sand-silt mixtures
		SC	Clayey sands, sand-clay mixtures
FINE-GRAINED SOILS (50% or more of material is smaller than No. 200 sieve size.)			
SILTS AND CLAYS Liquid limit less than 50%		ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
		OL	Organic silts and organic silty clays of low plasticity
SILTS AND CLAYS Liquid limit 50% or greater		MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
		CH	Inorganic clays of high plasticity, fat clays
		OH	Organic clays of medium to high plasticity, organic silts
HIGHLY ORGANIC SOILS		PT	Peat and other highly organic soils

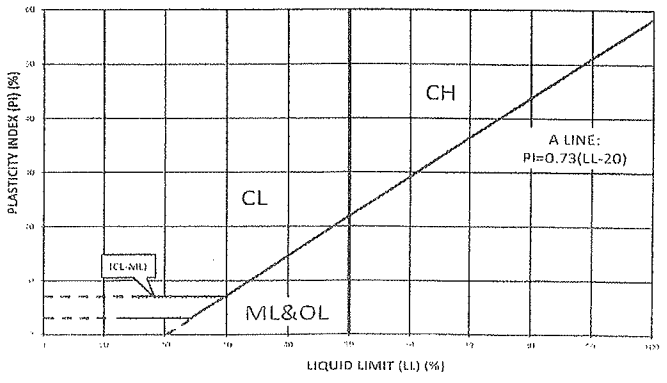
LABORATORY CLASSIFICATION CRITERIA

GW	$C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_C = \frac{D_{30}}{D_{10} \times D_{60}}$ between 1 and 3	
GP	Not meeting all gradation requirements for GW	
GM	Atterberg limts below "A" line or P.I. less than 4	Above "A" line with P.I. between 4 and 7 are borderline cases requiring use of dual symbols
GC	Atterberg limts above "A" line or P.I. greater than 7	
SW	$C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_C = \frac{D_{30}}{D_{10} \times D_{60}}$ between 1 and 3	
SP	Not meeting all gradation requirements for GW	
SM	Atterberg limits below "A" line or P.I. less than 4	Limits plotting in shaded zone with P.I. between 4 and 7 are borderline cases requiring use of dual symbols
SC	Atterberg limits above "A" line with P.I. greater than 7	

Determine percentages of sand and gravel from grain-size curve. Depending on percentage of fines (fraction smaller than No. 200 sieve size), coarse-grained soils are classified as follows:

Less than 5 percent ..... GW, GP, SW, SP
More than 12 percent ..... GM, GC, SM, SC
5 to 12 percent ..... Borderline cases requiring dual symbols

PLASTICITY CHART



DOCUMENT QUALIFICATIONS

I. GENERAL RECOMMENDATIONS/LIMITATIONS

CGC, Inc. should be provided the opportunity for a general review of the final design and specifications to confirm that earthwork and foundation requirements have been properly interpreted in the design and specifications. CGC should be retained to provide soil engineering services during excavation and subgrade preparation. This will allow us to observe that construction proceeds in compliance with the design concepts, specifications and recommendations, and also will allow design changes to be made in the event that subsurface conditions differ from those anticipated prior to the start of construction. CGC does not assume responsibility for compliance with the recommendations in this report unless we are retained to provide construction testing and observation services.

This report has been prepared in accordance with generally accepted soil and foundation engineering practices and no other warranties are expressed or implied. The opinions and recommendations submitted in this report are based on interpretation of the subsurface information revealed by the test borings indicated on the location plan. The report does not reflect potential variations in subsurface conditions between or beyond these borings. Therefore, variations in soil conditions can be expected between the boring locations and fluctuations of groundwater levels may occur with time. The nature and extent of the variations may not become evident until construction.

II. IMPORTANT INFORMATION ABOUT YOUR GEOTECHNICAL ENGINEERING REPORT

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes. While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical engineering study conducted for a civil engineer may not fulfill the needs of a construction contractor or even another civil engineer. Because each geotechnical engineering study is unique, each geotechnical engineering report is unique, prepared *solely* for the client. *No one except you* should rely on your geotechnical engineering report without first conferring with the geotechnical engineer who prepared it. *And no one - not even you* - should apply the report for any purpose or project except the one originally contemplated.

READ THE FULL REPORT

Serious problems have occurred because those relying on a geotechnical engineering report did not read it all. Do not rely on an executive summary. Do not read selected elements only.

A GEOTECHNICAL ENGINEERING REPORT IS BASED ON A UNIQUE SET OF PROJECT-SPECIFIC FACTORS

Geotechnical engineers consider a number of unique, project-specific factors when establishing the scope of a study. Typical factors include: the client's goals, objectives, and risk management preferences; the general nature of the structure involved, its size, and configuration; the location of the structure on the site; and other planned or existing site improvements, such as access roads, parking lots, and underground utilities. Unless the geotechnical engineer who conducted the study specifically indicates otherwise, *do not rely on a geotechnical engineering report* that was:

- not prepared for you,
- not prepared for your project,
- not prepared for the specific site explored, or
- completed before important project changes were made.

Typical changes that can erode the reliability of an existing geotechnical report include those that affect:

- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light industrial plant to a refrigerated warehouse,
- elevation, configuration, location, orientation, or weight of the proposed structure,
- composition of the design team, or project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes - even minor ones - and request an assessment of their impact. *CGC cannot accept responsibility or liability for problems that occur because our reports do not consider developments of which we were not informed.*

SUBSURFACE CONDITIONS CAN CHANGE

A geotechnical engineering report is based on conditions that existed at the time the geotechnical engineer performed the study. *Do not rely on a geotechnical engineering report* whose adequacy may have been affected by: the passage of time; by man-made events, such as construction on or adjacent to the site; or by natural events, such as floods, earthquakes, or groundwater fluctuations. *Always* contact the geotechnical engineer before applying the report to determine if it is still reliable. A minor amount of additional testing or analysis could prevent major problems.

MOST GEOTECHNICAL FINDINGS ARE PROFESSIONAL OPINION

Site exploration identifies subsurface conditions only at those points where subsurface tests are conducted or samples are taken. Geotechnical engineers review field and laboratory data and then apply their professional judgement to render an opinion about subsurface conditions throughout the site. Actual subsurface conditions may differ - sometimes significantly - from those indicated in your report. Retaining the geotechnical engineer who developed your report to provide construction observation is the most

effective method of managing the risks associated with unanticipated conditions.

A REPORT'S RECOMMENDATIONS ARE NOT FINAL

Do not over-rely on the confirmation-dependent recommendations included in your report. *Those confirmation-dependent recommendations are not final*, because geotechnical engineers develop them principally from judgement and opinion. Geotechnical engineers can finalize their recommendations *only* by observing actual subsurface conditions revealed during construction. *CGC cannot assume responsibility or liability for the report's confirmation-dependent recommendations if we do not perform the geotechnical-construction observation required to confirm the recommendations' applicability.*

A GEOTECHNICAL ENGINEERING REPORT IS SUBJECT TO MISINTERPRETATION

Other design team members' misinterpretation of geotechnical engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer confer with appropriate members of the design team after submitting the report. Also retain your geotechnical engineer to review pertinent elements of the design team's plans and specifications. Constructors can also misinterpret a geotechnical engineering report. Confront that risk by having CGC participate in prebid and preconstruction conferences, and by providing geotechnical construction observation.

DO NOT REDRAW THE ENGINEER'S LOGS

Geotechnical engineers prepare final boring and testing logs based upon their interpretation of field logs and laboratory data. To prevent errors or omissions, the logs included in a geotechnical engineering report should *never* be redrawn for inclusion in architectural or other design drawings. Only photographic or electronic reproduction is acceptable, *but recognize that separating logs from the report can elevate risk.*

GIVE CONSTRUCTORS A COMPLETE REPORT AND GUIDANCE

Some owners and design professionals mistakenly believe they can make constructors liable for unanticipated subsurface conditions by limiting what they provide for bid preparation. To help prevent costly problems, give constructors the complete geotechnical engineering report, *but* preface it with a clearly written letter of transmittal. In that letter, advise constructors that the report was not prepared for purposes of bid development and that the report's accuracy is limited; encourage them to confer with the geotechnical engineer who prepared the report (a modest fee may be required) and/or to conduct additional study to obtain the specific types of information they need or prefer. A prebid conference can also be valuable. *Be sure constructors have sufficient time* to perform additional study. Only then might you be in a position to give constructors the best information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions.

READ RESPONSIBILITY PROVISIONS CLOSELY

Some clients, design professionals, and constructors do not recognize that geotechnical engineering is far less exact than other engineering disciplines. This lack of understanding has created unrealistic expectations that have led to disappointments, claims, and disputes. To help reduce the risk of such outcomes, geotechnical engineers commonly include a variety of explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineer's responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

ENVIRONMENTAL CONCERNS ARE NOT COVERED

The equipment, techniques, and personnel used to perform an *environmental* study differ significantly from those used to perform a *geotechnical* study. For that reason, a geotechnical engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated environmental problems have led to numerous project failures.* If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk management guidance. *Do not rely on an environmental report prepared for someone else.*

OBTAIN PROFESSIONAL ASSISTANCE TO DEAL WITH MOLD

Diverse strategies can be applied during building design, construction, operation, and maintenance to prevent significant amounts of mold from growing on indoor surfaces. To be effective, all such strategies should be devised for the *express purpose* of mold prevention, integrated into a comprehensive plan, and executed with diligent oversight by a professional mold prevention consultant. Because just a small amount of water or moisture can lead to the development of severe mold infestations, many mold prevention strategies focus on keeping building surfaces dry. While groundwater, water infiltration, and similar issues may have been addressed as part of the geotechnical engineering study whose findings are conveyed in this report, the geotechnical engineer in charge of this project is not a mold prevention consultant; ***none of the services performed in connection with the geotechnical engineer's study were designed or conducted for the purpose of mold prevention. Proper implementation of the recommendations conveyed in this report will not of itself be sufficient to prevent mold from growing in or on the structure involved.***

RELY ON YOUR GEOTECHNICAL ENGINEER FOR ADDITIONAL ASSISTANCE

Membership in the Geotechnical Business Council (GBC) of Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk confrontation techniques that can be of genuine benefit for everyone involved with a construction project. Confer with CGC, a member of GBC, for more information.

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Geotechnical Business Council
of the Geoprofessional Business Association
8811 Colesville Road, Suite G 106
Silver Spring, MD 20910

Legend

Denotes Boring Location

N

Scale: Reduced

Notes

1. Boring locations are approximate
2. Soil borings performed by Soil Essentials in October 2021

Date: 10/2021	CGC, Inc.	**Soil Boring Location Map** **Warner Park Crayon Playground** **Madison, WI**
Job No. C21051-13		